

# 6th SC7 Working Group Meeting WG3/TG3 Introduction to Discussions on Pile Design and PT4 Drafting Work

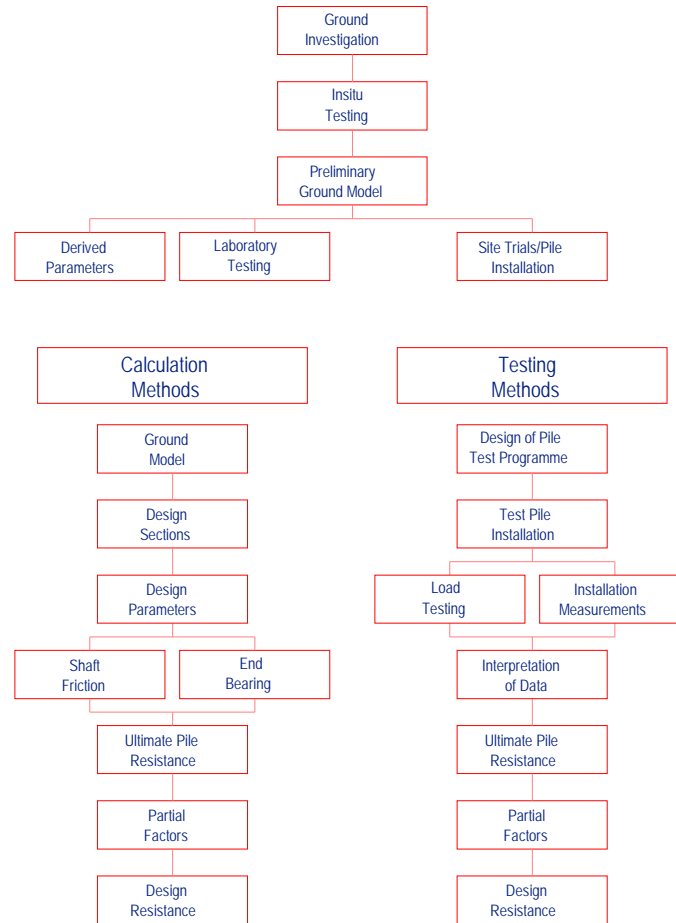
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## What am I going to talk about?

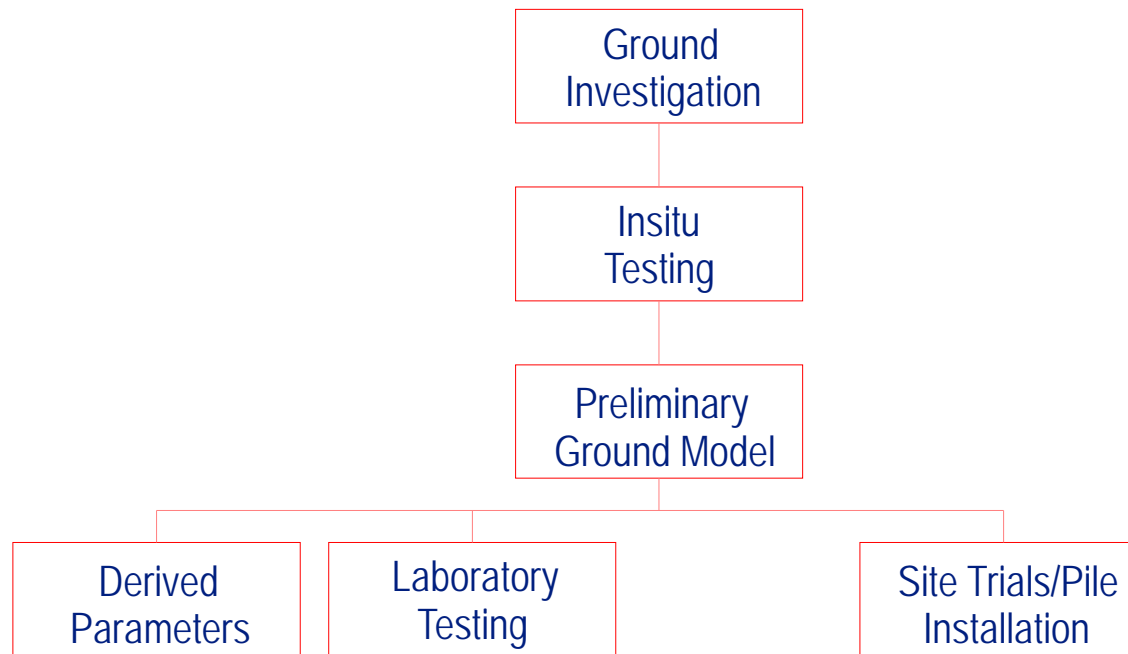
- Taken over Chair of WG3/TG3
- Hand Over from EG7 to PT4
- Pile Design - Bearing Capacity
  - Focus not on the differences across Europe
  - Highlight the similarities
- Interface between WG3/TG3 and PT4
- Key topics that need to be included in new draft
- Some possible discussion topics

# Flow Chart – Pile Bearing Capacity



# Ground Investigation

- All pile design starts with site investigation, insitu testing and laboratory testing to a greater or lesser extent

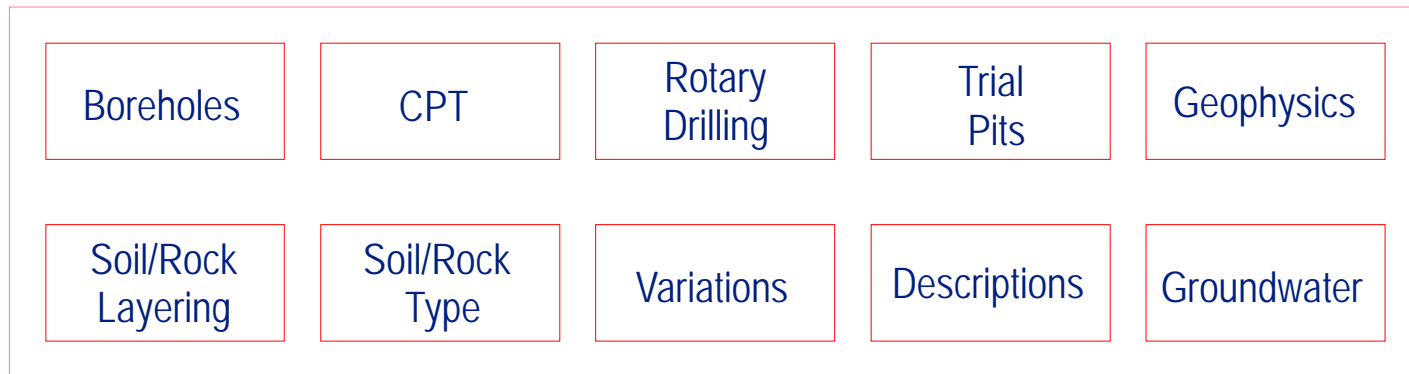


## Ground Investigation

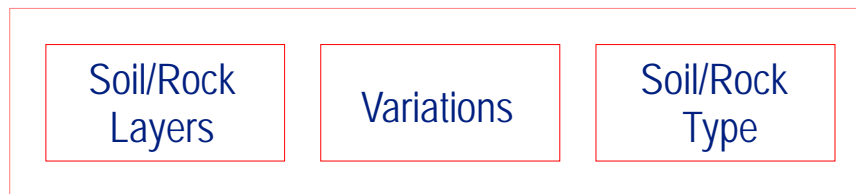
- Site investigation is used to establish:
  - Geometrical constraints
  - Soil layering
  - Soil and rock types
  - Variations
  - Groundwater conditions
- Also used to determine basic soil parameters

# Ground Investigation

- Site investigation is used to establish geometrical constraints, soil layering, variations and groundwater conditions



- But also a preliminary Ground Model



## Ground Investigation

- But site investigation could also include:
  - Site trials and prototype pile installation
  - Installation of piles for testing
  - Observation of pile spoil from bored piles
  - Drive blows for driven piles
  - Drive energy [wave analysis] for driven piles
  - Dynamic load testing
  - Static load testing

## Ground Investigation

- Insitu testing is used to determine basic soil parameters

CPT  $q_c$

SPT  $N_{60}$

PMT  $p_L$

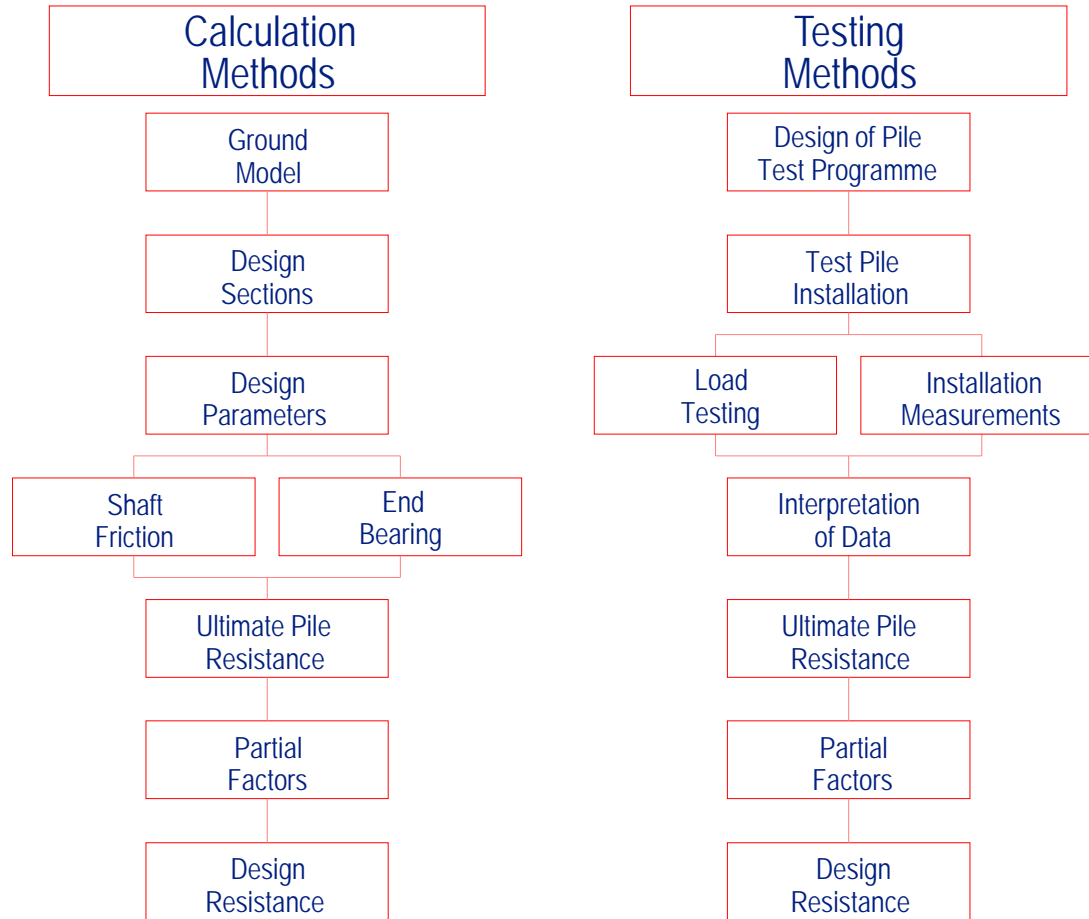
- There are differences across Europe on what we do with the basic soil parameters and other test data
- But all approaches are being used successfully somewhere
- PT4 need to remain inclusive and not rule out any particular approach as being better or worse



## Pile Design

- Pile design can broadly be divided into two methods:
  - Calculation based on insitu or laboratory test results
  - Design based on pile load testing
- Both options are legitimate and acceptable
- Both methods are being used successfully across Europe
- But PT4 must not show preference for either option

# Pile Design By Calculation or Testing



# Pile Design By Calculation

- All design methods require a ground model

Soil/Rock  
Layers

Variations

Soil/Rock  
Type

- These are simplified to provide representative design sections or multiple profiles [Borehole or CPT profiles]

Simplified  
Sections

Assumed  
Soil Properties

Groundwater  
Conditions

## Pile Design By Calculation

- The differences in the design methods in use across Europe relate mainly to how insitu or laboratory test data is used
- But they all have in common the derivation of pile shaft stress  $f_s$  and end bearing pressure  $q_b$
- These can be via derived soil properties  $c'$ ,  $\phi'$ ,  $C_u$  or UCS
- Or by direct correlation from CPT  $q_c$ , SPT  $N_{60}$  or PMT  $p_L$
- Or by using experience [tables related to  $q_c$  or  $C_u$ ]

# Pile Design By Calculation



- The similarity is that all methods end up with  $f_s$  and  $q_b$ , whether this is the German EA-Pfähle, UK  $\phi'$  and  $C_u$  methods, the French PMT methods or other methods in use elsewhere in Europe

## Pile Design By Calculation

- The resulting  $f_s$  and  $q_b$  are used to **calculate** the ultimate pile shaft  $R_s$  and end bearing  $R_b$  resistances
- A model factor is often applied to derive the characteristic values of the shaft  $R_{s,k}$  and end bearing  $R_{b,k}$  resistances

Shaft  
Resistance

$$\sum f_{s,k} A_s$$

Base  
Resistance

$$q_{b,k} A_b$$

## Pile Design By Calculation

- The characteristic resistance is taken equal to the sum of the characteristic pile shaft  $R_{s,k}$  and end bearing  $R_{b,k}$  resistances

Pile  
Resistance

$$R_{ult} = R_{s,k} + R_{b,k}$$

- The calculation method is common in many European countries
- PT4 need to recognise that the calculation method is possibly the most common pile design approach in Europe

# Pile Design By Testing

- The other main design methods are based on pile installation measurements or pile load testing
- All methods requires a preliminary ground model from the GI





# Pile Design By Testing

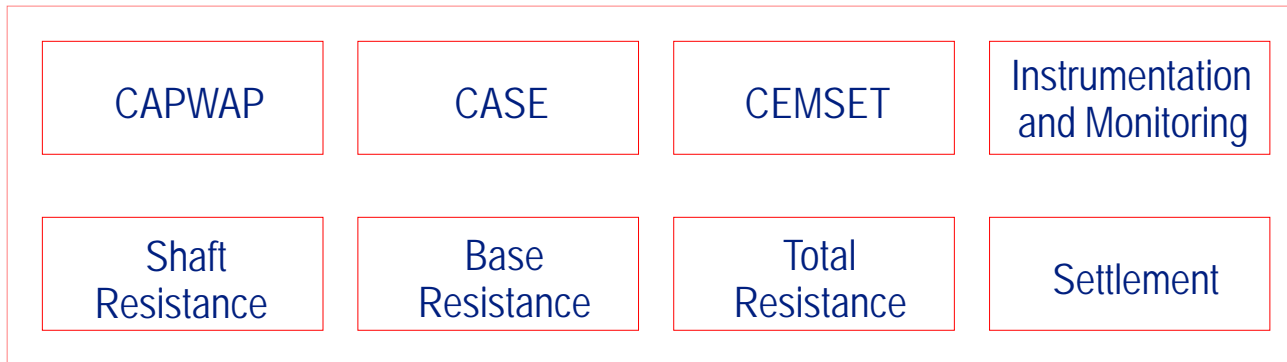
- Pile load testing



- Testing can be static or dynamic load tests, or wave analysis of the energy measurements during installation, or analysis of the measured drive blows
- Load tests can also be instrumented to provide more data

# Pile Design By Testing

- Pile load testing requires interpretation



- Instrumented load tests can provide more data including separation of the shaft and end bearing components
- Most interpretation will just give a single total resistance

## Pile Design By Testing

- There are also difficulties in determining the ultimate pile resistance from the test data
- Some options:

$$R_{ult} = R_{test, max}$$

$$R_{ult} = R_{test, \Delta = 10\% Dia}$$

$$R_{ult} = R_{extrapolation to \infty}$$

- This is an area that PT4 may need some guidance from WG3/TG3

## Pile Design By Testing

- Design by pile testing is reliable under certain circumstances
- **BUT** difficulties can arise due to:
  - Large variations in ground geometry
  - Variations in pile loads
  - Changes in vertical effective stress after testing
  - Ground heave or negative shaft friction
- To deal with these, the Designer needs to revert to **calculation**

## Design Approach and Factors

- Currently, there are very big differences across Europe in the choice of Design Approach, model, correlation and resistance factors adopted for pile design
- PT4 have a major difficulty here in meeting the requirement to harmonise and simplify
- Recommendations from PT1/PT2 are to adopt the resistance factor approach [RFA] for pile design
- Whatever PT4 recommend, they must be inclusive of all methods currently being used

## Pile Design By Calculation

- The design resistances  $R_{c;d}$  or  $R_{t;d}$  are obtained from the characteristic end bearing and shaft friction by using partial resistance factors

$$R_{c;d} = \left[ \frac{R_{b;k}}{\gamma_b} + \frac{R_{s;k}}{\gamma_s} \right] \text{ or } \left[ \frac{R_{c;k}}{\gamma_t} \right] \quad R_{c;k} = R_{b;k} + R_{s;k}$$

- PT4 need guidance on values for  $\gamma$
- Can values be simplified, or extended?
- UK use different values dependent on amount of load testing

## Pile Design From Static Load Tests

- The characteristic resistance is obtained directly from static load testing by applying correlation factors  $\xi$  using the following

$$R_{c;k} = \text{Min} \left[ \frac{\text{Mean } R_{c;m}}{\xi_1} \right] \text{ or } \left[ \frac{\text{Minimum } R_{c;m}}{\xi_2} \right]$$

- The design resistance  $R_{c;d}$  can then be obtained using partial resistance factors

$$R_{c;d} = \left[ \frac{R_{c;k}}{\gamma_t} \right]$$

- PT4 need guidance on values for  $\xi$

## Pile Design From Dynamic Impact Tests

- The characteristic resistance can also be obtained from dynamic impact test data using the following similar relationship:

$$R_{c;k} = \text{Min} \left[ \frac{\text{Mean } R_{c;m}}{\xi_5} \right] \text{ or } \left[ \frac{\text{Minimum } R_{c;m}}{\xi_6} \right]$$

- An additional model factor  $\gamma_{Rd}$  is also required:
  - 0.85 when using signal matching (CAPWAP)
  - 1.10 when the test includes pile head displacement
  - 1.20 if no measurement of pile head displacement
- PT4 need guidance on values for  $\xi$  and  $\gamma_{Rd}$



## Pile Design From Ground Test Results

- The characteristic resistance can also be obtained from empirical relationships with ground test results (such as CPT) using the following similar relationship:

$$R_{c;k} = \text{Min} \left[ \frac{\text{Mean } R_{c;cal}}{\xi_3} \right] \text{ or } \left[ \frac{\text{Minimum } R_{c;cal}}{\xi_4} \right]$$

- Values for  $\xi_3$  and  $\xi_4$  depend on the number of ground test results with values decreasing as the number of profiles increases
- PT4 need guidance on values for  $\xi$

## Interface between WG3/TG3 and PT4

- PT4 can essentially ignore anything that WG3/TG3 says or recommends as there is no contractual linkage between the two
- **BUT** PT4 need to convince WG3/TG3 to accept any draft at the end of the process
- What I hope is that PT4 will work with WG3/TG3 [and the other task groups] and maintain some liaison throughout the process
- There are a large number of areas where WG3/TG3 could provide support to PT4

## Key Issues to be included in the PT4 Draft

- Pile design by calculation as the primary pile design method
- Guidance on model factors
- Confirm values for resistance factors
  - But maintain different factors for different pile types
  - Different factors for different load test validation
  - Factors to consider settlement control
- Confirm values for correlation factors
- Recommendations must be inclusive of all current methods

## Some Possible Discussion Topics 1

- Is it necessary to include derivation of shaft friction  $f_s$  and end bearing pressure  $q_b$  as informative annexes to EN 1997?
  - These can be via derived soil properties  $c'$ ,  $\phi'$ ,  $C_u$  or UCS
  - Or by direct correlation from CPT  $q_c$ , SPT  $N_{60}$  or PMT  $p_L$
  - Or by using experience [tables related to  $q_c$  or  $C_u$ ]
- Surely these are well covered by text books, guidance documents, research papers and other NCCI

## Some Possible Discussion Topics 2

- Should interpretation of static and dynamic pile load testing be included?
- Are simple rules such as load corresponding to a pile settlement equal to 10% of the diameter/size a reasonable definition?
- Or should we extrapolate to infinite movement as this is more mathematically correct?

## Some Possible Discussion Topics 3

- Should horizontal loads on piles be designed using partial load and resistance factors, or is it better to treat as a serviceability problem using unfactored loads and parameters?
- Load factors would be applied to the effect of the actions

## Some Possible Discussion Topics 4

- Should pile groups be designed using partial load and resistance factors, or is it better to treat large pile groups as a serviceability problem using unfactored loads and parameters?
- What are the difficulties in designing combined raft and pile foundations?

## Some Possible Discussion Topics 5

- Should structural resistance of piles be covered by EN 1997 as this is not adequately dealt with by EN 1992?
- What are the difficulties in designing long slender piles to avoid buckling and bending problems in very soft ground?
- Do we need to adopt the MFA rather than RFA in this case?